



# PERFORMANCE EVALUATION OF ASPHALT PAVEMENT MIXES IN IDAHO CONTAINING HIGH PERCENTAGES FOR RECYCLED ASPHALT PAVEMENT (RAP)

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**University of Idaho**

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# Outline

- Background
- Objectives
- Laboratory Characterization
- Findings

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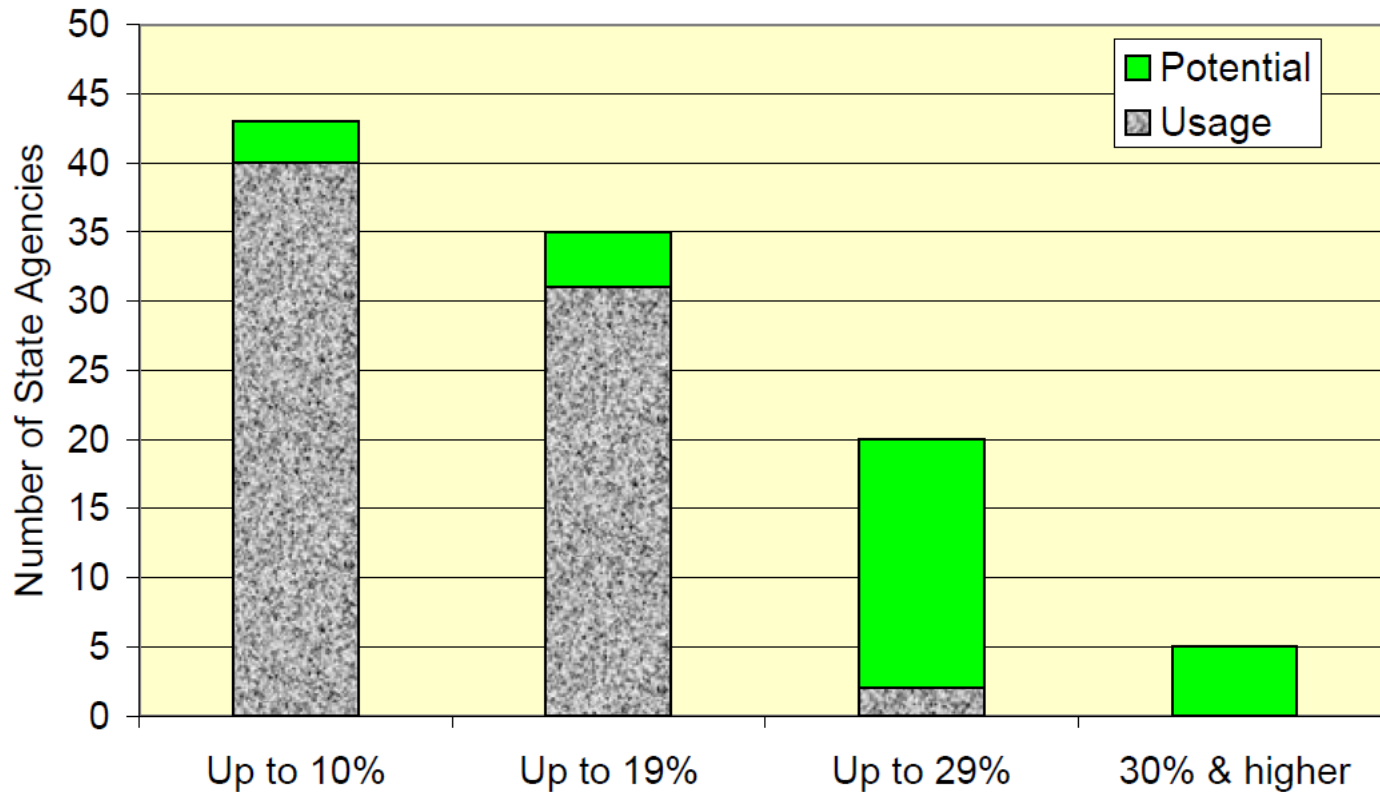
- **Background**
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# Background

- Benefits of using RAP in HMA
  - Economics
    - Aggregates
    - Binder
  - Environment
    - Resources
    - Petroleum
    - Landfill
    - Energy
    - Emission

# Background

- Status of the use of RAP in HMA



# Mix Design-Virgin Binder Selection

- ITD Binder Adjustment
  - Replacement  $\leq 17\%$ . No adjustment
  - $17\% < \text{Replacement} \leq 30$ . One grade lower
  - Replacement  $> 30\%$ . Blending chart.
    - Based on assumption of complete blending between RAP binder and virgin binder

# Dynamic Modulus

- Dynamic modulus **increased** with increasing RAP percentage, and RAP significantly affects dynamic modulus values at intermediate and high temperature (Li 2008, McDaniel 2012, Qazi 2011)

# Performance-Rutting

- **Consensus Conclusion:**
  - Rutting resistance **increased** as the increase of percent of RAP (Hajj 2009, Qazi 2011, Santos 2010, Yu 2010, Colbert 2012)
  - Aged RAP binder increase the stiffness of mixture



# Fatigue Cracking

- Most studies show that RAP mixtures had **reduced** fatigue life or more brittle behavior (Huang 2011, Shu 2008, Yu 2010, NCHRP 9-12)
- A few studies, however, showed that mixtures with RAP had **better** fatigue life (Santos 2010, Hajj 2009, McDaniel 2012)
- Fatigue life of stiffer mixes depends on the thickness of layer (Sousa 1998, Hassan 2009)



# Thermal Cracking



- Fracture Energy (Li 2008)
  - Decrease as RAP content increased, indicating **lower** low-temperature fracture resistance
- Fracture temperature (Hajj 2011)
  - Thermal stress retained specimen test (TSRST) test
  - similar TSRST fracture temperature between 0 and 15% RAP mixes
  - several degree warmer for 50% RAP mixes , indicating **decreased** thermal cracking resistance
- Using soft binder could help improve thermal cracking resistance

# Moisture Susceptibility

- Mixtures with RAP could have **acceptable** resistance to moisture damage, or addition of antistripping additive could help mixtures with RAP gain TSRs above 0.80 (Hajj 2009, NCHRP 9-46, Yu 2010, Loria 2011)



# Background

- We can not wait for 20 years to see the performance
- Need to determine the performance before pavement with high RAP percentage is built
- Key is to select materials properties from lab to relate to field performance for performance evaluation and also mix design

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# Objective

- **Verify the guideline by ITD on the use of RAP in HMA to lead to same performance in the laboratory**
- **Evaluate the effect of RAP on pavement performance**

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- **Laboratory Characterization**
- Findings

# Material Procurement

- Plant Loose Mixes and Field Cores
  - US95 Garwood to Sagle, 30% RAP by binder replacement
- Lab Mixes
  - Binder:
    - PG58-28 (Control), PG52-34
  - Aggregates:
    - Nominal Maximum Size is 19mm



# RAP Characterization

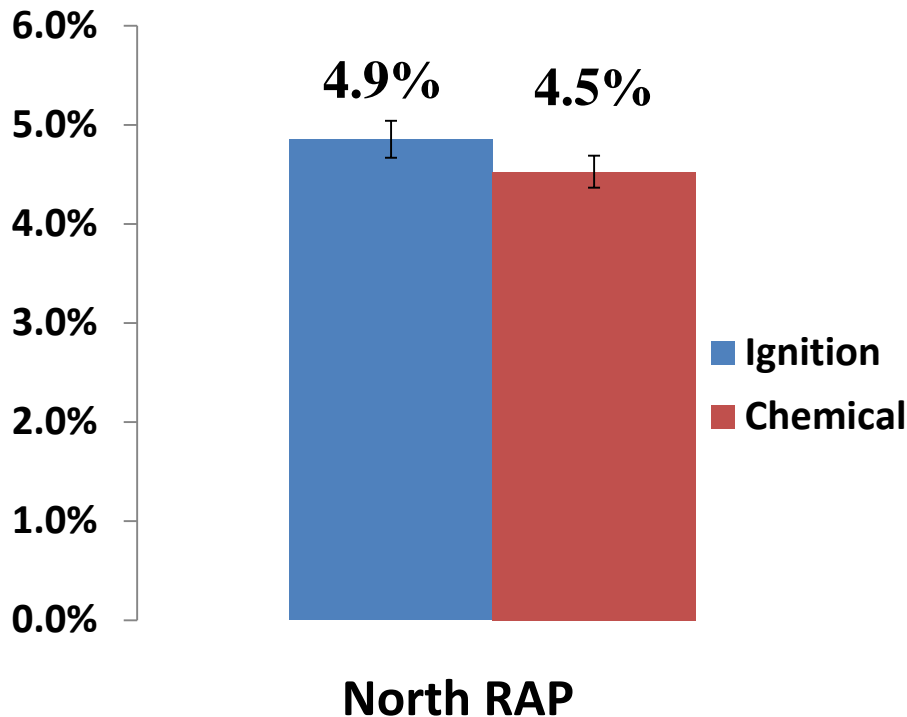
- Binder Content
- RAP Aggregate Gradation
- Bulk Specific Gravity of RAP Aggregate
- PG of Extracted RAP Binder

# RAP Characterization

- **Fractionated**
  - Coarse RAP and fine RAP are separated by No.4 Screen
    - 0.53:0.47 for the North RAP
- **Recombined after homogenization in a concrete mixer**

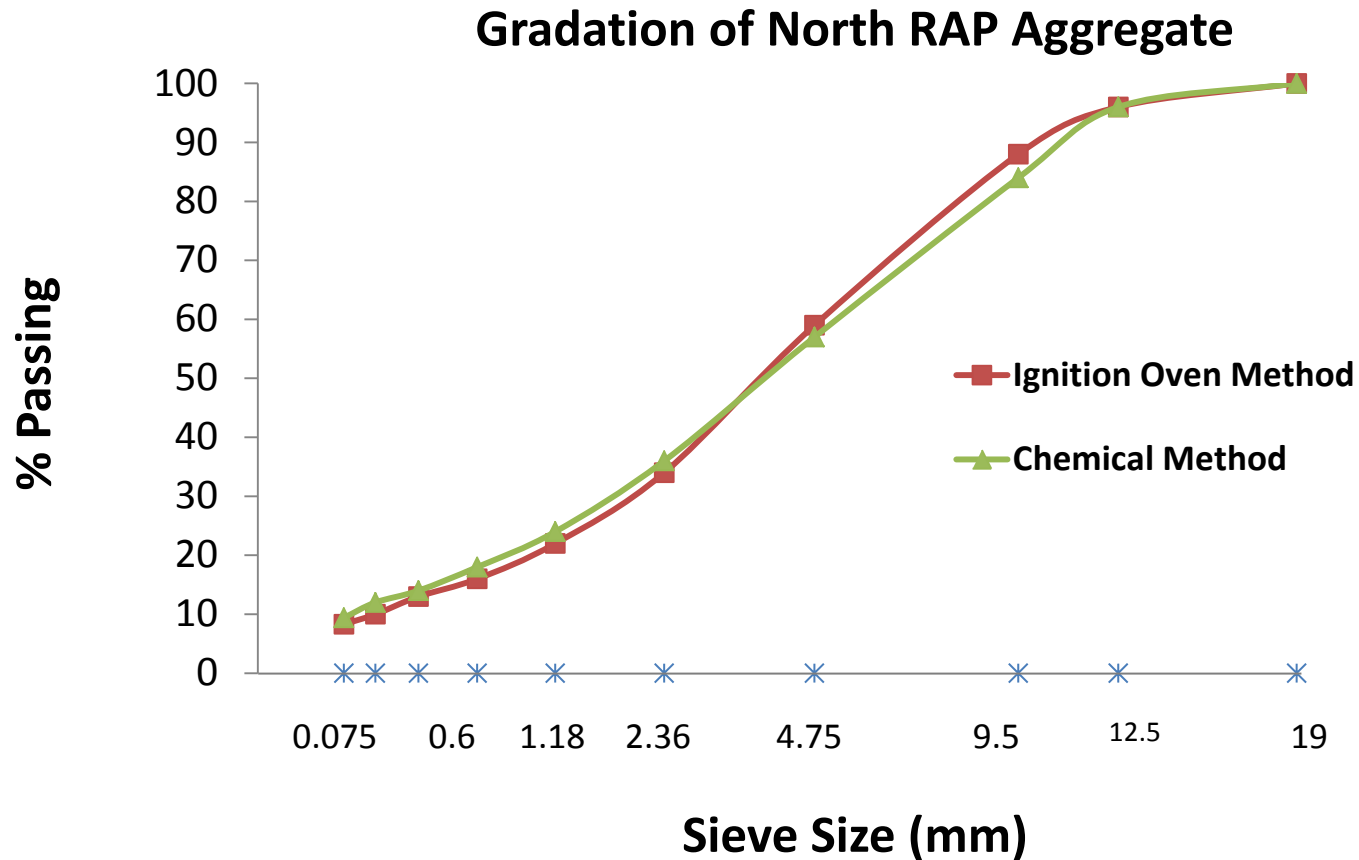
# RAP Binder Content

- Ignition Oven (AASHTO T308)
- Chemical Extraction (AASHTO T164 )



# Gradation of RAP Aggregate

- AASHTO T30 “Mechanical Analysis of Extracted Aggregate”



# Bulk Specific Gravity of RAP Aggregate

- **Ignition Oven : AASHTO T308**
  - **Coarse Aggregate: AASHTO T85**
  - **Fine Aggregate: IT 144**

North RAP Aggregate	1	2	3	Average	Std	COV
Coarse RAP aggregate	2.604	2.604	2.611	2.606	0.004	0.15%
Fine RAP aggregate	2.618	2.628	2.635	2.627	0.009	0.33%
Combined	2.619					

# Results of PG of Extracted Binder

- Chemical Extraction and Recovery:
  - AASHTO T164-11 & AASHTO T170
- RAP Binder: PG 75.8-23.6

	PG of Recovered North RAP binder					
	1	2	3	Average	Std	COV
High Temperature	76.9	74.9	75.5	<b>75.8</b>	1.0	1.3%
Low Temperature	-22.7	-24.6	-23.6	<b>-23.6</b>	1.0	4.2%

# Mix Design

- **Lab Mixes**

- Four different RAP percentages

- 0, 17, **30**, and 50% (N0, N17, N30 and N50)

- Duplicate field mix in terms of aggregate gradation

- US-95, Garwood to Sagle, Chilo STG

- Class of Mixture

- 3/4", SP5, Traffic 10-30 (ESALs)

# PG of Blended Binder for Mixes

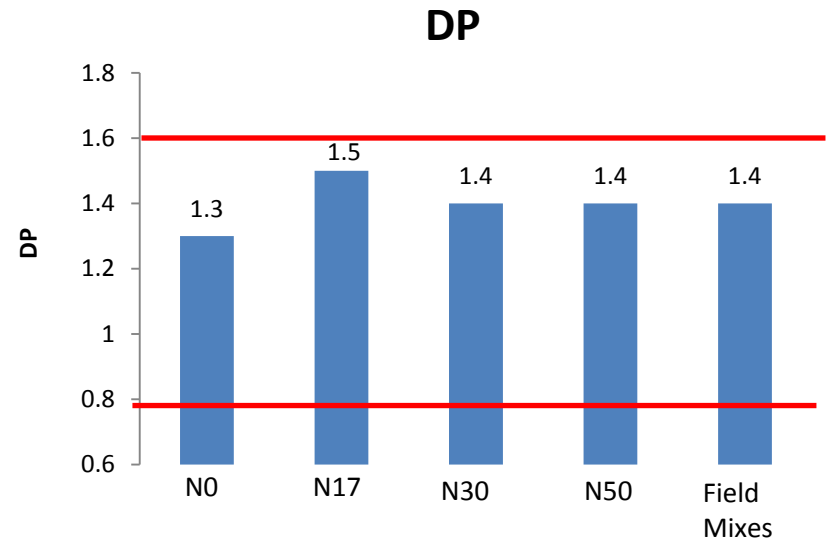
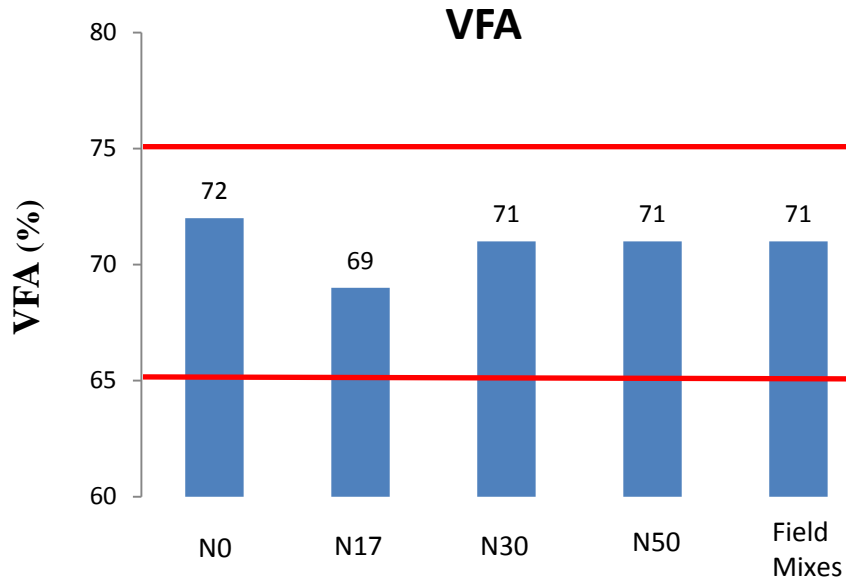
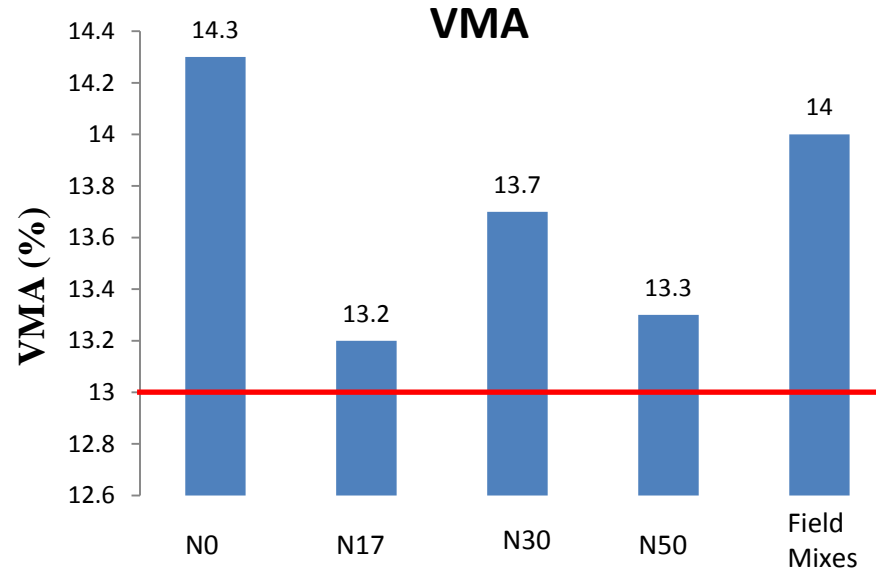
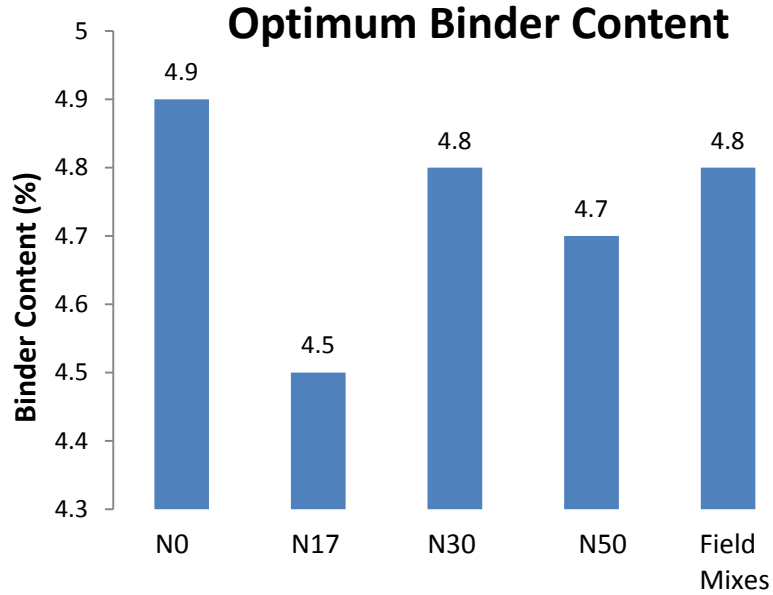
- Assuming 100% blending between the RAP binder and virgin binder

<b>% RAP</b>	<b>Virgin Binder</b>	<b>RAP binder</b>	<b>Blended Binder</b>	<b>Target PG of binder</b>
<b>0</b>	<b>58-28</b>	<b>-----</b>	<b>58-28</b>	<b>58-28</b>
<b>17</b>	<b>58-28</b>	<b>75.8-23.6</b>	<b>61.0-27.3</b>	
<b>30</b>	<b>52-34</b>		<b>59.1-30.9</b>	
<b>50</b>	<b>52-34 (40-34)</b>		<b>63.9-28.8</b>	





# Results of Mixes

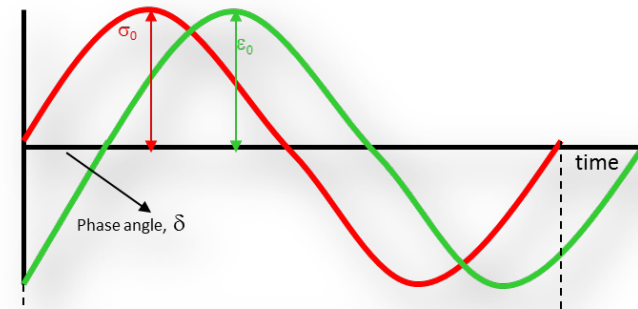


# Lab Performance Evaluation

- Modulus
- Rutting
- Fatigue Resistance
- Low Temperature Thermal Cracking

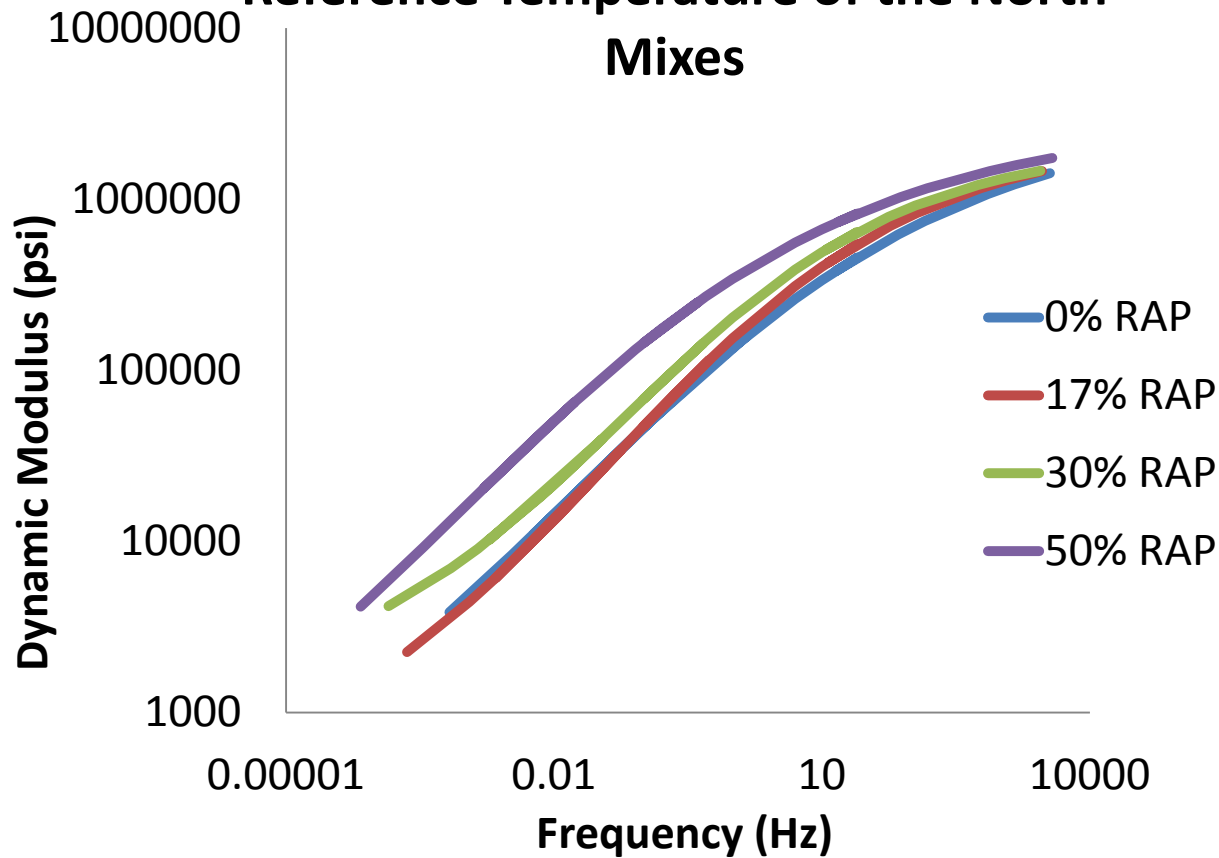
# Dynamic Modulus Test ( $E^*$ )

- Sample Preparation for  $E^*$ 
  - Mixing
  - Short term aging 140 F, 16hour aging
  - 2-2.5 hours aging at compaction temperature
  - Compaction
  - Core and cutting with air voids within 6.5%-7.5%
  - Testing temperatures ( 40° F, 70° F, 100° F, 130° F)
  - Loading frequencies(0.1Hz, 0.5Hz,1Hz, 5Hz, 10Hz, 25Hz).



# E\*- Master Curves-Mixes

**Dynamic Modulus Master Curves at 70° F**  
**Reference Temperature of the North**  
**Mixes**

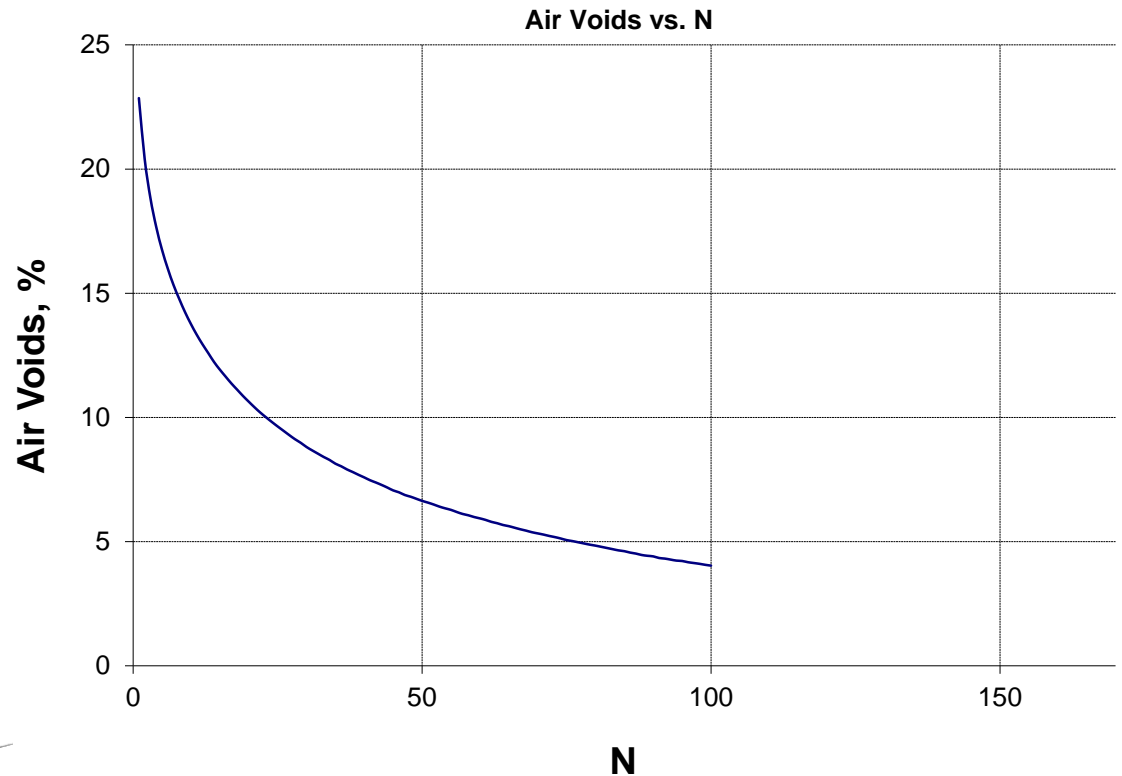
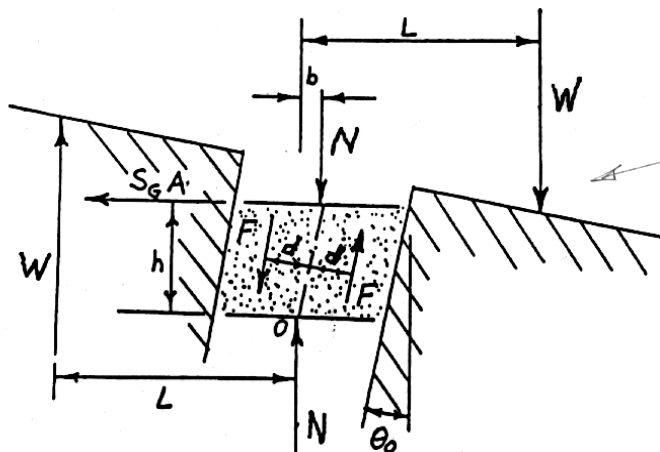


% RAP	Virgin Binder
0	58-28
17	58-28
30	52-34
50	52-34 (40-34)

# Gyratory stability (GS)-Rutting

$$GS = \sum_{NG1}^{N_{design}} S_e \times d_e$$

$$S = \frac{R \cdot e}{A \cdot h}$$



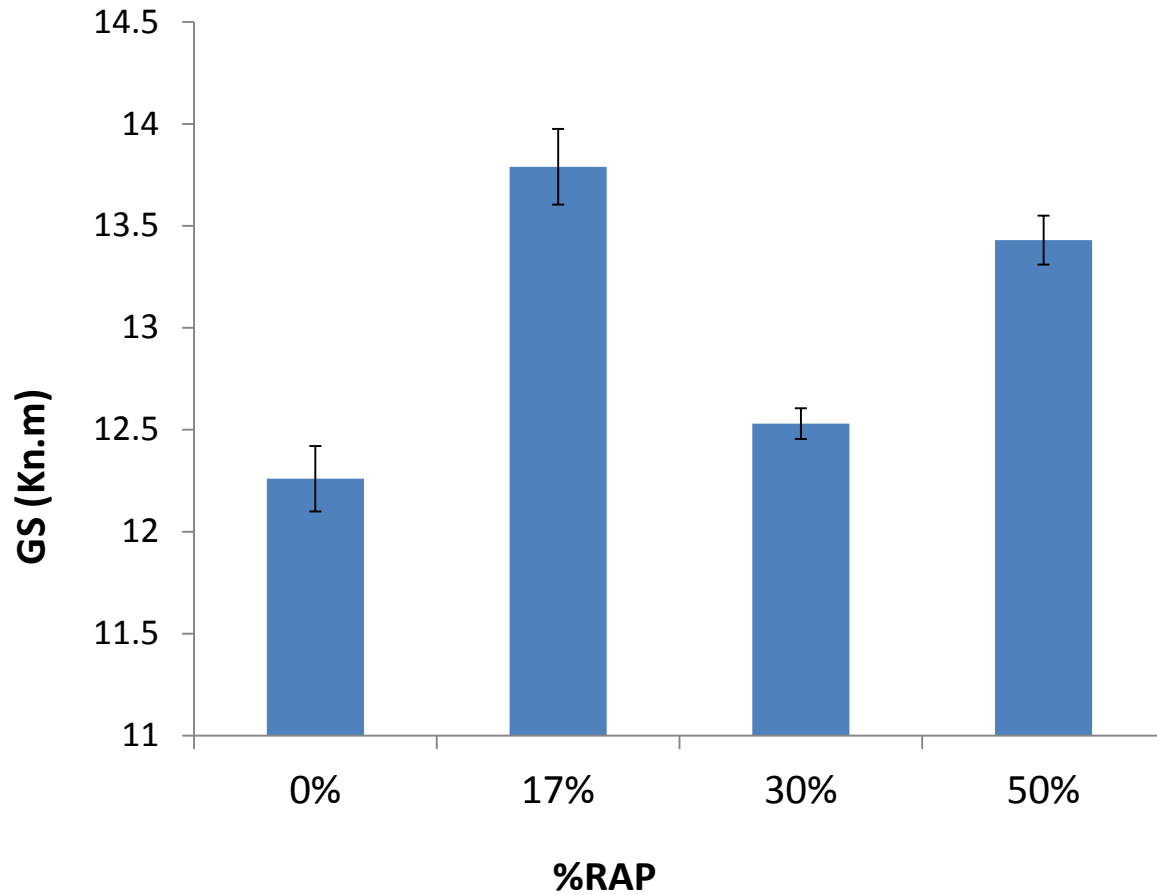
R = the resultant ram force

E = the average eccentricity for a given gyration cycle

A = the sample cross section, and

h = the sample height at any gyration cycle.

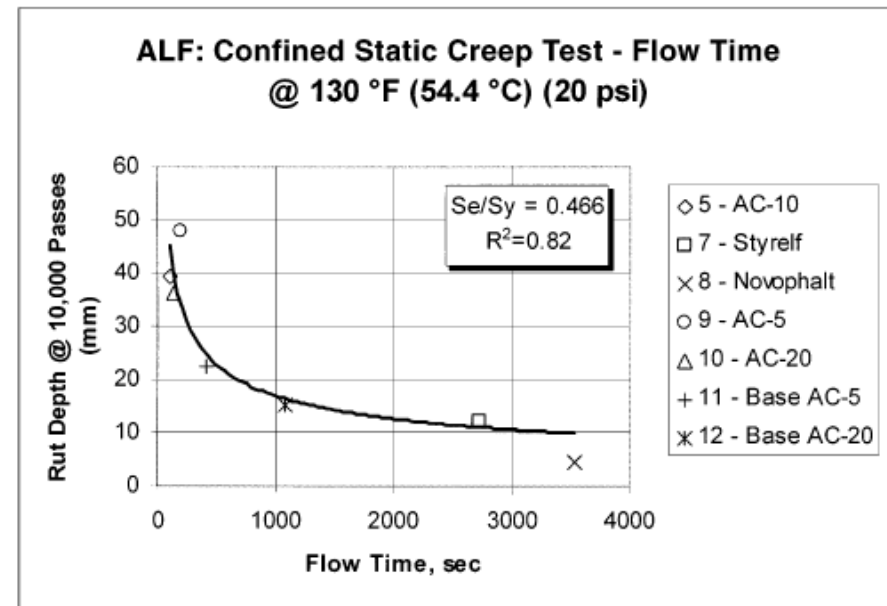
# Gyratory stability (GS) - Rutting



% RAP	Virgin Binder
<b>0</b>	<b>58-28</b>
<b>17</b>	<b>58-28</b>
<b>30</b>	<b>52-34</b>
<b>50</b>	<b>52-34 (40-34)</b>

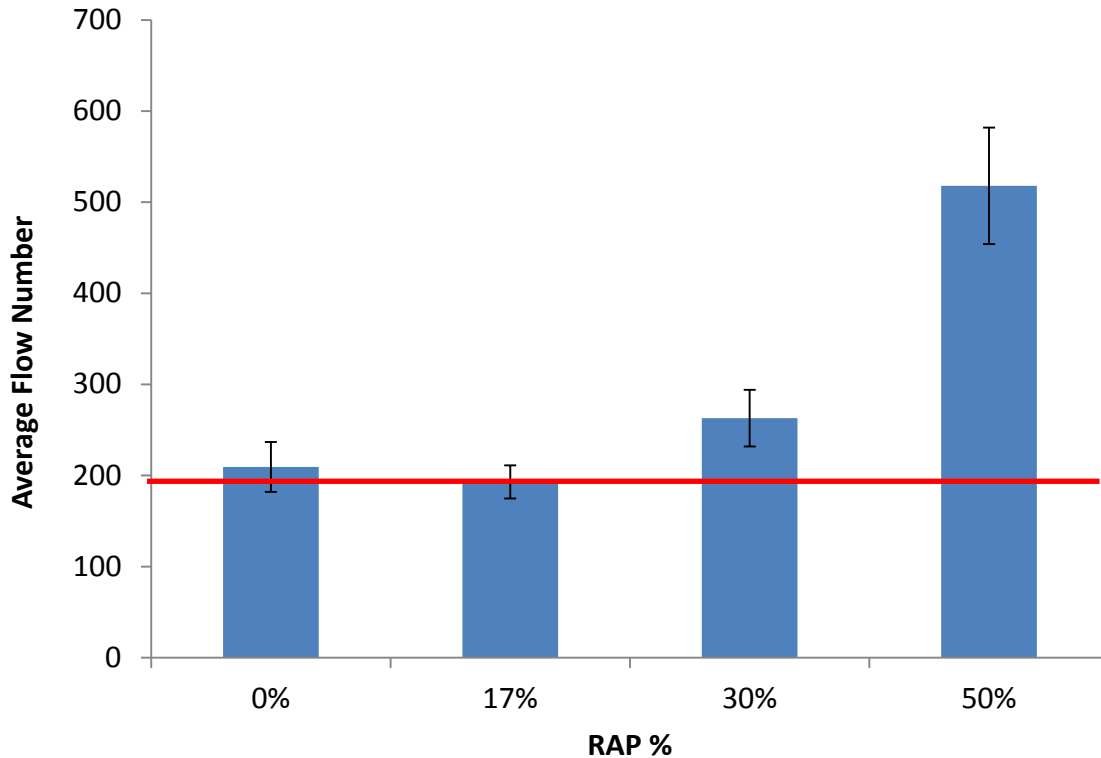
# Flow Number Rutting

- Laboratory Tests
  - Rutting (flow number) – repeated load @ high temperature



\*NCHRP Report 465

# Flow Number - Rutting



% RAP	Virgin Binder
<b>0</b>	<b>58-28</b>
<b>17</b>	<b>58-28</b>
<b>30</b>	<b>52-34</b>
<b>50</b>	<b>52-34 (40-34)</b>

Design Traffic, Million ESAL	HMA <sup>1</sup>	WMA <sup>1</sup>
< 3	--	--
3 to < 10	50	30
10 to < 30	190	105
> 30	740	415



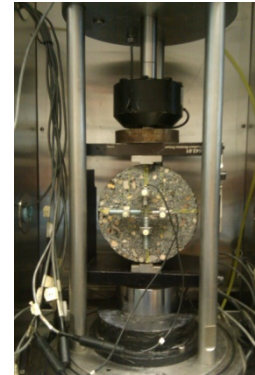
# Fatigue Performance Test

- For fatigue, test methods in the lab can include

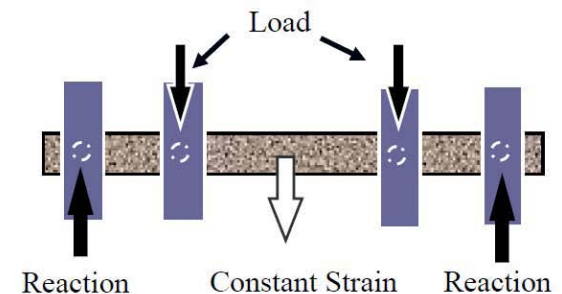
- Stiffness



- Indirect tensile strength

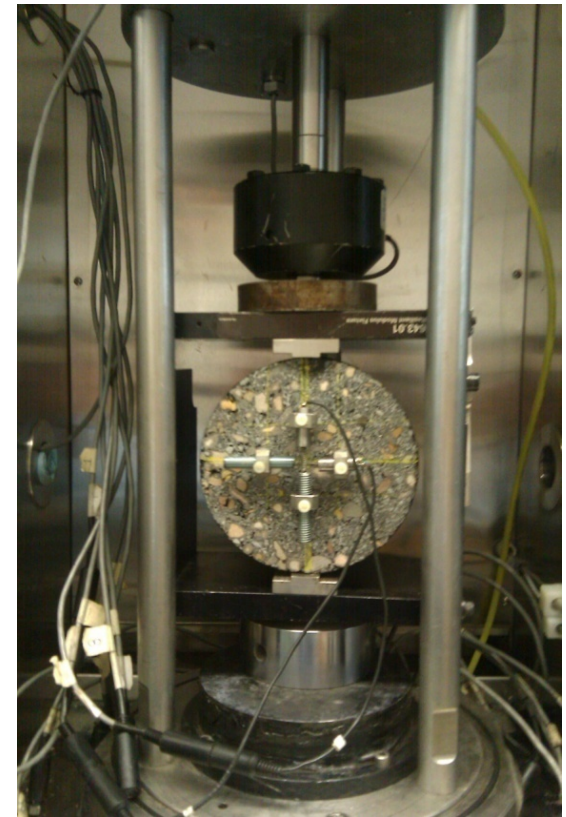


- Beam fatigue



# Fatigue Resistance

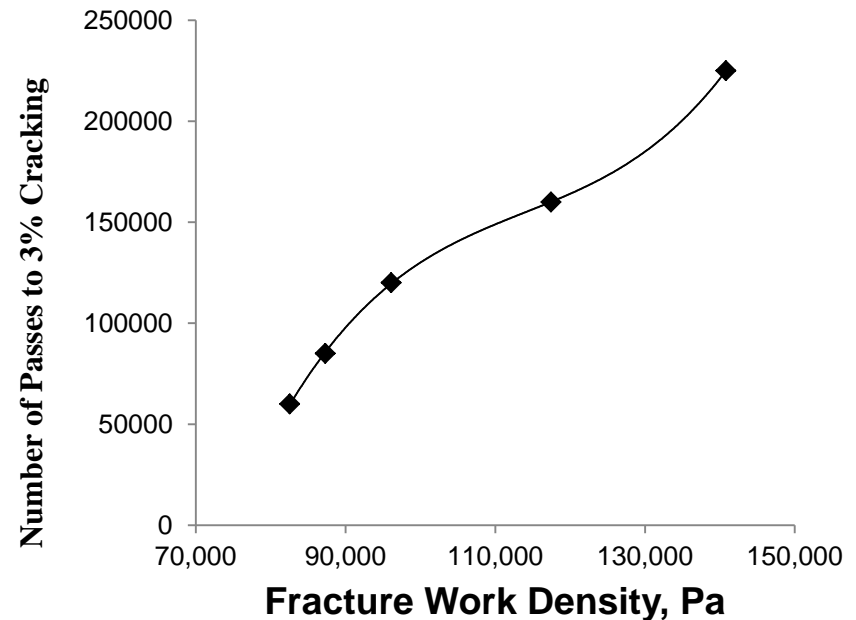
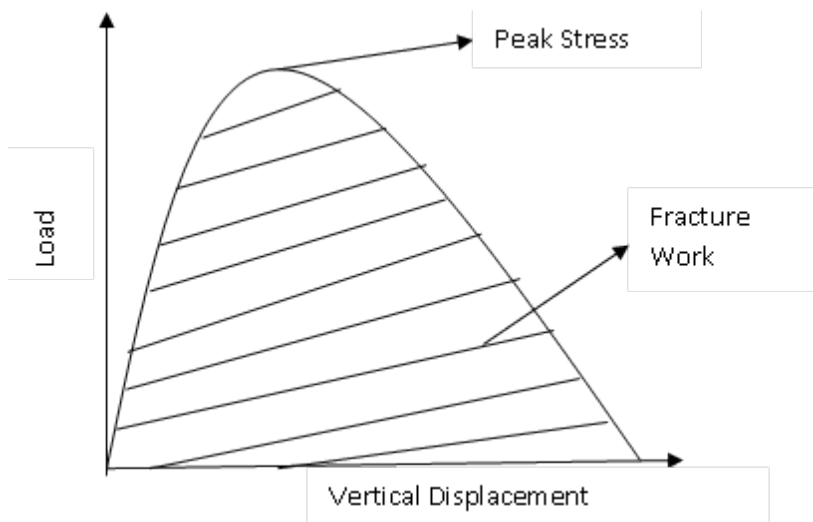
- **Long term aging**
  - 5 days at 185°F
- **Test temperature**
  - Temperature: 68°F
  - Displacement Control: 2inch/min
- **Properties**
  - Fracture Work Density
  - Vertical Failure Deformation



# Fracture Work Density

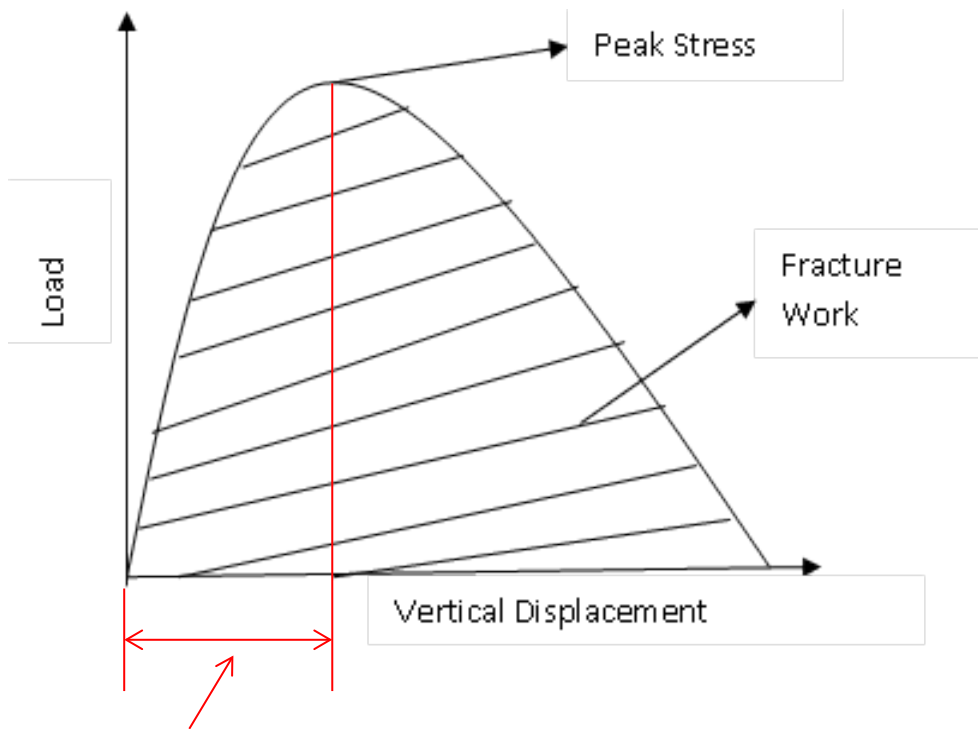


- Bottom-up fatigue cracking - fracture work from Indirect Tensile test at 68°F (Wen et al. 2011)



# Vertical Failure Deformation

- Top-down cracking – vertical failure deformation (Wen et al. 2013) 12 out of 15 pair pavements match

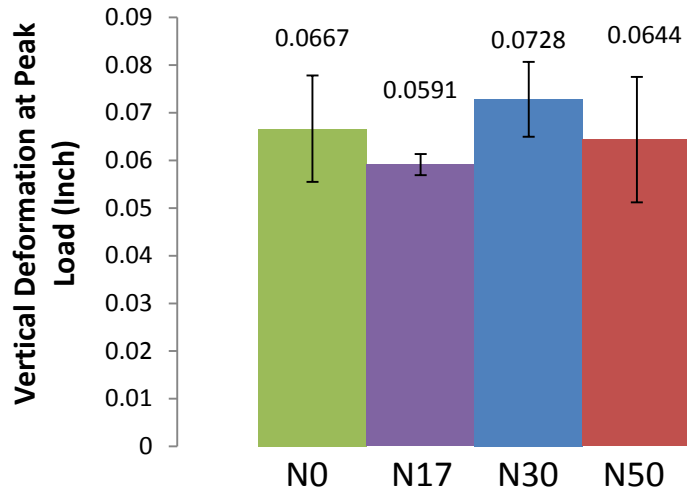
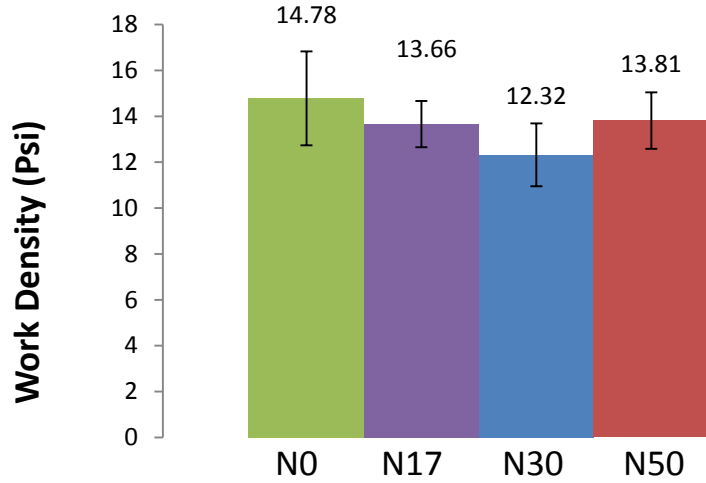


Vertical Failure Deformation





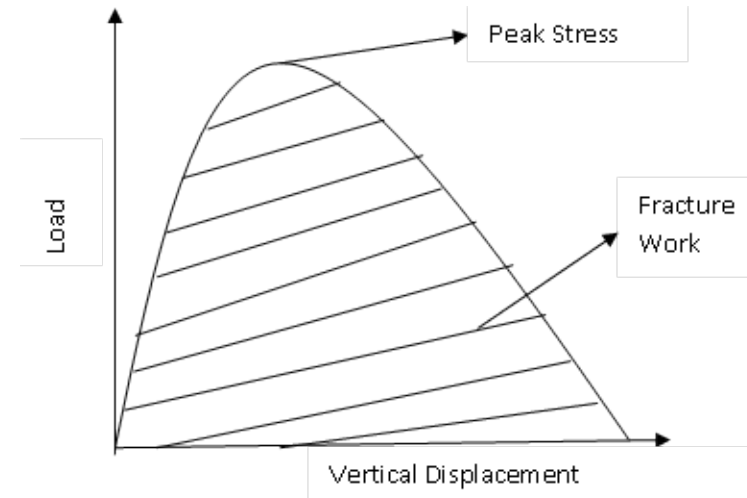
# Fatigue Results



% RAP	Virgin Binder
<b>0</b>	<b>58-28</b>
<b>17</b>	<b>58-28</b>
<b>30</b>	<b>52-34</b>
<b>50</b>	<b>52-34 (40-34)</b>

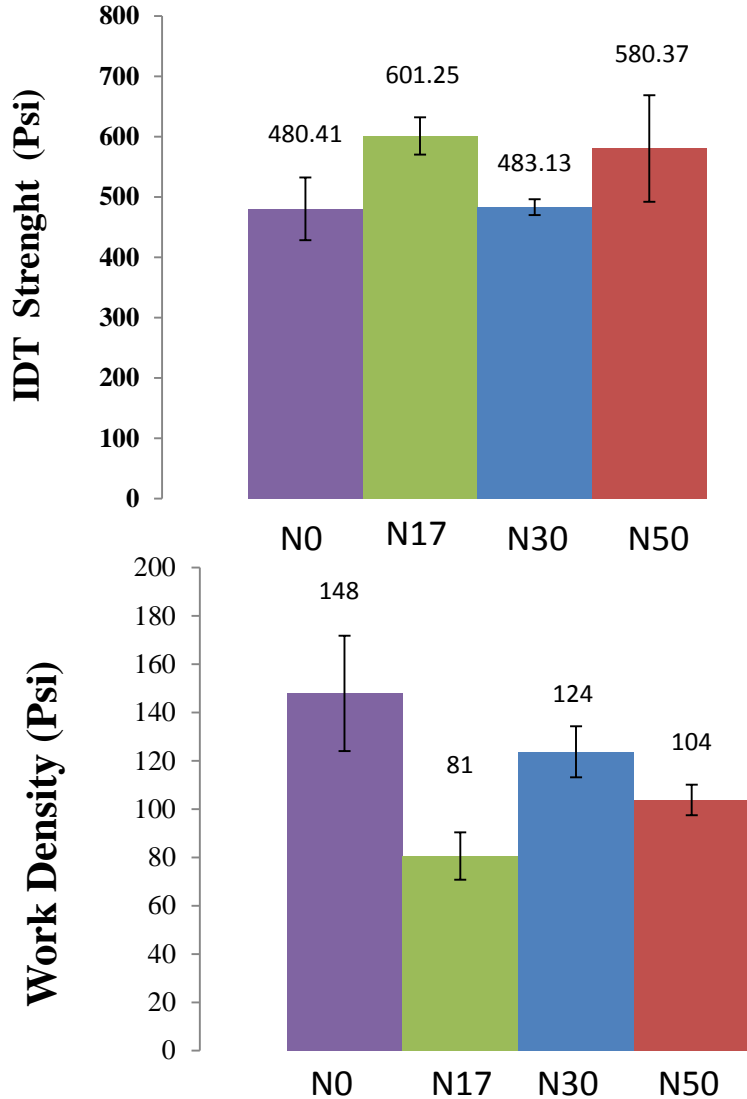
## 5.3 Low Temperature Thermal Cracking

- AASHTO T322
  - “Standard Method of Test for Determining the Creep Compliance and Strength of Hot-Mix Asphalt (HMA) Using the Indirect Tensile Test Device”



- IDT Strength Test
  - Temperature: 14°F
    - Fracture Work Density Correlates with Thermal Cracking
      - Wen et al. 2013, 15 out of 19 pair pavements match

# Results of Low Temperature Cracking



<b>% RAP</b>	<b>Virgin Binder</b>
<b>0</b>	<b>58-28</b>
<b>17</b>	<b>58-28</b>
<b>30</b>	<b>52-34</b>
<b>50</b>	<b>52-34 (40-34)</b>

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# Findings

- With the increase of RAP percentage
  - Stiffness increases
  - Rutting resistance increases
  - Fatigue cracking resistance is not affected
  - Low temperature cracking resistance is affected
- The low temperature cracking resistance can be improved by change of PG grade or mix design
- Further verification is needed (South Idaho Mix)

# **Acknowledgements**

The team would like to thank ITD for sponsoring this research and their support during the study



# **Washington Center for Asphalt Technology (WCAT)**

**Haifang Wen, PhD, PE, Director  
Assistant Professor  
Washington State University**



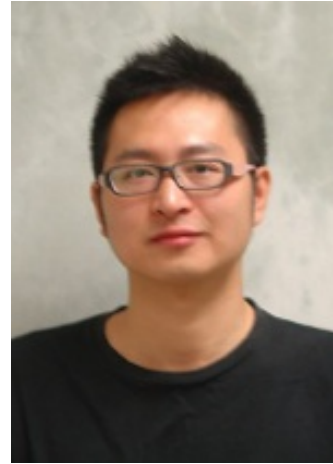
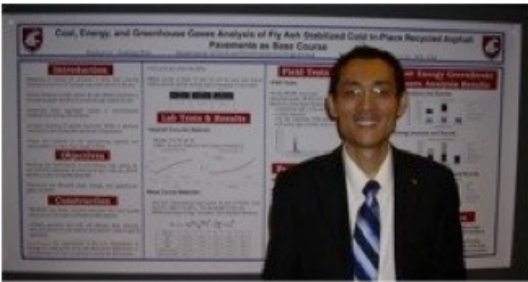
# Background

- Established through partnership between
  - Washington State Department of Transportation (WSDOT),
  - Washington Asphalt Paving Association (WAPA),  
and
  - Washington State University (WSU)
- Funding also contributed by National Science Foundation (NSF)
- Website: [wcat.cee.wsu.edu](http://wcat.cee.wsu.edu)

# Members



# Graduate Students



# WCAT Activities

- Education
  - Undergraduate and graduate students
- Industry services
  - Mix design and verification
  - Studies
- Research and development
  - NCHRP 09-49A, 04-36
  - FHWA EAR
  - National Science Foundation
  - WSDOT, ITD, WisDOT, Counties
  - University Transportation Centers
  - Industries

# Laboratory Experiments

- WCAT is AASHTO accredited
  - Mix design
  - Mix verification
- Binder Tests
  - Extraction and recovery
  - Asphalt Content of Compacted Bituminous Mixtures using Ignition Oven or Solvent
  - Dynamic Shear Rheometer
  - Bending Beam Rheometer
  - Rolling Thin Film Oven
  - Pressure Aging Vessel
  - Rotational Viscometer (Brookfield)





# Laboratory Experiments

- Mix performance tests
  - Dynamic Modulus Test - stiffness
  - Static Creep Test (Flow Time) - rutting
  - Repeated Load Test (Flow Number) – rutting
  - Indirect Tensile Test – fatigue and thermal cracking
  - Modified Lottman – moisture damage
  - Studded tire simulator



**Thanks!**

**Questions?**